



Characterization of concrete cubes by Sorptivity-de-sorptivity test

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The quantity of water present in the concrete matrix controls many fresh and hardened properties of concrete such as workability, compressive strengths, permeability and water tightness, durability and weathering, drying shrinkage and potential for cracking. Thus the limiting and controlling the amount of water present in concrete matrix as well as cement paste is important for both constructability and an extension service life of concrete infrastructures. The successful key for making durable concrete is to limit its ability to transport fluids like water. In order to devise realistic testing methods, that determine the ability of concrete to withstand water penetration requires an understanding of water mobility. In order to build durable oriented and practicable concrete structures, it is needed to be able to accurately predict the water sorptivity, water de-sorption, relationship between water sorptivity-de-sorptivity, water diffusion coefficient-sorptivity, water diffusion coefficient-moisture content, and moisture content-time duration within the concrete structures. Therefore, there is a need to quantify the sorptivity-de-sorptivity coefficient in concrete cubes which is of the most important factor in the concrete industries. The present research work is made an attempt to interpret the concrete sorptivity-de-sorptivity coefficient in ordered to characterize the different concrete mixtures design for in case of concrete cubes. Thus the objectives of this present research are such as: this research will examine the influence of concrete ingredients on the results of water sorptivity-de-sorptivity performed on concrete cubes with different mixtures proportion in which slump, and w/c ratio value is varied with constant compressive strength as in the First case and compressive strength, and w/c ratio value varied with constant slump as in the Second case. Seventy-two concrete cubes (100 mm^3) with Grades of concrete ranges from 25 to 40 N/mm² were prepared and evaluate the water sorptivity effect in designed different mixtures type. As from this research work that, it's possible to establish power type of equation between de-sorptivity coefficient and square root of time in designed mixtures type. The de-sorptivity coefficient is predominantly increased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's also confirmed from the results that, the de-sorptivity coefficient is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of de-sorptivity coefficient with square root of time is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type. It's possible to establish polynomial type of equation between sorptivity-de-sorptivity coefficient ratio and square root of time in designed mixtures type. The sorptivity-de-sorptivity coefficient ratio is predominantly decreased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's also confirmed from the results that, the sorptivity-de-sorptivity coefficient ratio is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of sorptivity-de-sorptivity coefficient ratio with square root of time is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type. From this research work that, it's possible to establish polynomial type of equation between water diffusion coefficient and moisture content with constant higher concrete compressive strength and varied slump value for in case of designed mixtures type. Finally, from this research work that, it's possible to establish power type of equation between water diffusion coefficient and moisture content with varied compressive strength and constant slump value for in case of designed mixtures type. The water diffusion coefficient is increased at an initial stage with lesser moisture content for in case of lower compressive strength and constant slump value and goes on reduced with pre-dominantly increased moisture content. But it's also confirmed from the results that, the water diffusion coefficient is slightly decreased at initial stage with lesser moisture content and goes on reduced with lower moisture content for in case of for in case of higher compressive strength and constant slump value. Whereas in the case of constant higher compressive strength and varied slump value, the variation of water diffusion coefficient with moisture content is slightly increased at an initial stage with lower moisture content and goes on decreases with increased moisture content for in case of constant higher compressive strength and varied slump value for in case of designed mixtures type. It's possible to establish relationship between moisture content and time duration for in designed mixtures type. The moisture content is predominantly decreased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's confirmed from the results that, the moisture content is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of moisture content with time duration is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type. From this research work that, it's possible to establish power

type of equation between water diffusion coefficient and sorptivity coefficient in designed mixtures type. The water diffusion coefficient is lesser at an initial stage when the rate of absorption (sorptivity) is lesser at an initial stage for in case of all mixtures type. It's also confirmed from the results that, the water diffusion coefficient is co-related with sorptivity coefficient; in turn the average variation of water diffusion coefficient with sorptivity coefficient is slightly more for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of water diffusion coefficient with sorptivity coefficient is slightly higher in case of lower compressive strength and constant slump and goes on decreases with increased compressive strength for in case of designed mixtures type.

INTRODUCTION

The primary mechanisms which is responsible for the deterioration of concrete structures includes such as the corrosion of reinforcing steel, cracking due to shrinkage, freezing and thawing, and chemical attack. Water is the principal agent responsible for the deterioration/ the principal medium by which aggressive agents such as chloride /sulfate ions are transported into the concrete. Thus the water ingress and aggressive agent transport are the key factors which influence the long-term durability of concrete structure. There many transport processes in concrete which includes such as capillary absorption, diffusion, permeation, and convection respectively. Capillary absorption describes water uptake in un-saturated concrete [1] due to the capillary forces. Diffusion describes the transport of moisture/dissolved ions as a result of a concentration gradient [2]. Permeation describes the flow of a fluid (water/air) as a result of gravity or a pressure gradient [3]. Convection/advection is the process that describes the transport of a solute (chloride/sulfate ions) as a result of the bulk moving water [4]. The analysis of transport processes in many concrete structures is complicated which may be due to numerous factors involved in transport mechanisms. In fact that, the concrete infrastructures are exposed to salts due to either a marine environment exposure or the application of de-icing salts to pavements, bridge decks, or parking lots [5-6]. In addition to that, an evidence of salt deterioration has been reported in masonry structures [7], building stones [8], coastal structures [9] and concrete elements [10]. There are several mechanisms may be associated with de-icing salt damage which may include factors such as pressure that develops due to osmosis, crystallization, intermediate compounds, or the increase of the risk of frost damage due to the increase in the degree of saturation [11]. In addition to that, the de-icing salts are also responsible of chemical interaction within the concrete, resulting in leaching and decomposition of the hydrated cement products, accelerated concrete carbonation, or alkali-silica reaction [12]. As noted by researchers [13] that, the sorptivity is one of the important parameter which inform the durability performance of concrete structure. Sorptivity is depends on the composition of concrete mixture such as w/c ratio and the curing procedure. It's confirmed from the results, there is a clear (approximately linear) decrease of sorptivity values with distance from the upper surface of the element and also inform about the influence of the compaction method on the values of sorptivity. Sorptivity are measured by mass method or volumetric method in which concrete specimens are dried to the constant mass [14].In which volumetric method is based on measuring the volume of water which penetrates the concrete at given time under the capillary forces through the surface equal to cross-section of glass cylinders with scaled pipettes through which water flows. Some researcher's points out that concrete sorptivity in the structure is different than the one tested

with specimens being cured in a laboratory in the water or in other conditions as pointed by researcher [15].

It's clear from the investigators [16] that, the Blast furnace slag (BFS) and natural pozzolana (NP) have been widely used as a partial cement replacement in concrete construction due to their cost reduction, improvement of the ultimate mechanical, and durability properties. It is possible to obtain the same or better strength grades by replacing cement with BFS up to 30% in concrete. However, the use of NP content reduced the compressive strength. Lower capillary water absorption for BFS or NP substitution is observed. An attempt is made by investigators [17] that, in order to develop models that allow to predict sorptivity of concrete with recycled aggregate on the basis of the composition of the concrete is presented. It's clear the formulated models showed very good agreement of mean values and satisfactory compliance of the standard deviation of the results obtained from the simulation with the results obtained from the sorptivity tests. An extensive research is carried out by investigators [18] on the hygro-thermal performances of three types of zeolite-based humidity control building materials (ZBHCMS). The experimental results indicated that the humidity control performance of ZBHCMS is strongly affected by the porosity and the pore diameter. The environmental temperature and the RH have considerable influence on the adsorption performance of ZBHCMS and De-sorption performance of ZBHCMS is affected more strongly by the ambient RH. Furthermore, the moisture diffusivity of unsaturated concrete is expressed so many decades before by [19] as a non-linear function of pore humidity and their proposed analytical model was in the same work calibrated with experimental data. It's later extended this model by researcher [20] to include moisture capacity as a function of water-cement ratio, curing time, temperature and type of cement as the derivative of the adsorption isotherm. Thus there is need to investigate the effectiveness of rate of absorption (sorptivity) which is performed on the concrete cubes in order to establish relationship between sorptivity-de-sorptivity, de-sorptivity-time, water diffusion coefficient-sorptivity, water diffusion coefficient-moisture content, and moisture content ratio coefficient-time respectively.

RESEARCH OBJECTIVES

The water transport in a porous network like concrete is a complex criteria. This is due to the fact that, many different kinds of transport mechanisms in combination with various types of pores that typically appears in the same porous system. Therefore there is a need to study water transport mechanisms with different designed mixtures type in order to assess the sorptivity-de-sorptivity coefficient in concrete structures. The present research work is made an attempt to interpret the concrete water sorptivity-de-sorptivity coefficient, de-sorptivity-time, water diffusion coefficient-sorptivity, water diffusion coefficient-moisture content, and moisture content ratio coefficient-time respectively in ordered to characterize the different concrete mixtures design for in case of concrete cubes. Thus the objectives of this present research is to examine the influence of concrete ingredients on the results of concrete water sorptivity-de-sorptivity coefficient performed on concrete cubes with different mixtures proportion in which slump,

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and w/c ratio value is varied with constant compressive strength as in the First case and compressive strength, and w/c ratio value is varied with constant slump as in the Second case. Seventy-two concrete cubes (100 mm^3) with Grades of concrete ranges from 25 to 40 N/mm 2 were prepared and evaluate the concrete water sorptivity coefficient in concrete cubes.

EXPERIMENTAL PROGRAM

In the present research work, six different mixtures type were prepared in total as per BRE, 1988 [21] code standards with a concrete cubes of size (100 mm^3). Three of the mixtures type were concrete cubes (100 mm^3) with a compressive strength 40 N/mm 2 , slump (0-10, 10-30, and 60-180 mm), and different w/c (0.45, 0.44, and 0.43). These mixtures were designated as M1, M2, and M3. Another Three of the mixtures type were concrete cubes with a compressive strength (25 N/mm 2 , 30 N/mm 2 , and 40 N/mm 2), slump (10-30 mm), and different w/c (0.5 0.45, and 0.44). These mixtures were designated as M4, M5, and M6. The overall details of the mixture proportions were to be represented in Table.1-2. Twelve concrete cubes of size (100 mm^3) were cast for each mixture and overall Seventy-two concrete cubes were casted for six types of concrete mixture. The coarse aggregate used is crushed stone with maximum nominal size of 10 mm with grade of cement 42.5 N/mm 2 and fine aggregate used was 4.75 mm sieve size down 600 microns for this research work respectively.

De-sorptivity coefficient

It's a phenomenon whereby a substance is released from or through a surface and in fact the process is the opposite of sorption. De-sorption is a process which involves the liberation of both absorbed and adsorbed water molecules. As the term sorption in concrete technology is used to describe both absorption and adsorption so desorption should be considered the opposite process where water is released from a concrete surface. Therefore, de-sorptivity is defined as a measure of the rate at which concrete releases water into a drying environment. The drying of a saturated concrete surface will develop menisci within the pore structure creating capillary tension will influence water transport. Therefore a de-sorptivity coefficient can be obtained from measuring uniaxial drying from a concrete surface in a constant temperature and humidity environment. The variation of de-sorptivity coefficient (D_s) with square root of time (\sqrt{t}) for in case of designed mixtures type with their correlation equation as well as R^2 values is represented in (Table 3).

The de-sorption coefficient was found to be increased ($40 \text{ g/m}^2/\text{min}^{0.5}$) at initial time duration as when compared to longer time duration ($0.3 \text{ g/m}^2/\text{min}^{0.5}$) in all mixtures type (M1-M6). The de-sorptivity coefficient was varied may be due to temperature, humidity, location dependent, slump value, water to cement ratio, and pore structure degree of saturation. The desorption coefficient was the opposite phase of Sorptivity coefficient. The De-sorptivity coefficient was investigated in all mixtures type (M1-M6) at different time interval for up to 28 days. The desorption coefficient was the rate of decrease of water absorption at each and every time interval which was depends on environmental conditions such as temperature, humidity, pore structure, compactness of concrete, and mixture proportion. The desorption coefficient was carried out by simply exposed the concrete cubes to room temperature and noted their reduced weight at each time until it reaches equilibrium state. The variation in desorption coefficient was found to be varied in between ($D_{s5 \text{ min}} = 43.96 \text{ g/m}^2/\text{min}^{0.5}$, and $D_{s200.79 \text{ min}} = 0.312 \text{ g/m}^2/\text{min}^{0.5}$) for in case mixtures type (M1-M6) and ($D_{s5 \text{ min}} = 43.17 \text{ g/m}^2/\text{min}^{0.5}$, and $D_{s200.79 \text{ min}} = 0.286 \text{ g/m}^2/\text{min}^{0.5}$) for in case of mixtures type (M1-M3), as well as ($D_{s5 \text{ min}} = 44.75 \text{ g/m}^2/\text{min}^{0.5}$, and $D_{s200.79 \text{ min}} = 0.338 \text{ g/m}^2/\text{min}^{0.5}$) in mixtures type (M4-M6). Similarly the variation of average values of de-sorption coefficient, minimum, maximum, and standard deviation for in case of different mixtures type (M1-M6) is as represented in Table.4 respectively.

Variation of De-Sorptivity and Sorptivity coefficient

The sorptivity and de-sorptivity coefficient increases gradually at three stages which follows square root of time and linearly proportional to each other. It's observed from results that, the linearity proportional ranges between 0-50 min, 50-100 min, and 100-200 min. The ratio of sorptivity to de-sorptivity coefficient values range between 0.0023-0.15 at short and long time duration in all mixtures type (M1-M6). The sorptivity and de-sorptivity coefficient follows linearity of proportional, this may be due the fact that, both the coefficients directly proportional to cumulative water absorption (mass gain), mass loss, and inversely proportional to square root of time. Therefore, the Sorptivity coefficient is equal to de-sorptivity coefficient which follows linearity of proportion. The variation of Sorptivity to De-sorptivity ratio coefficient was evaluated at different time interval in all mixtures type (M1-M6) with their correlation equations and R^2 value as shown in Table.5. The ratio varies due environmental conditions and location. Actually the rate of absorption was not suddenly increased/decreased in turn depends on concrete matrix, and mixture proportion, but rate of absorption was increases gradually with time duration. Similarly, the rate of decrease of water from any structure was not so easy because the pore structure formation, compactness and if it's properly mixture designed. In fact, rate of desorption was very slow in all mixtures type. From this ratio, it's possible to predict time duration in any designed mixtures type in turn it's possible to interpret the particular mixture type characteristics such as compressive strength, slump, w-c ratio, Fine-coarse aggregate volume fraction, cement paste and concrete matrix. The variation in Sorptivity-desorption coefficient ratio was found to be varied in between ($S/D_{s5 \text{ min}} = 0.023$, and $S/D_{s200.79 \text{ min}} = 0.166$) for in case mixtures type (M1-M6) and ($S/D_{s5 \text{ min}} = 0.022$, and $S/D_{s200.79 \text{ min}} = 0.167$) for in case of mixtures type (M1-M3), as well as ($S/D_{s5 \text{ min}} = 0.024$, and $S/D_{s200.79 \text{ min}} = 0.166$) in mixtures type (M4-M6).

Relationship between water diffusion coefficient and moisture content

The water diffusion coefficient was increases at an initial time duration in all mixtures type (M1-M6) in which its ranged about $2.25 \text{ mm}^2/\text{min}$ at 5 min time duration with moisture content ($Mc = 1.07\%$). The diffusion coefficient-moisture content curve deviates nearer point at moisture content ($Mc = 1.9-2\%$) in almost all mixtures type at which the diffusion coefficient was about at least $1.072 \text{ mm}^2/\text{min}$. After time passes, the water diffusion coefficient was reached equilibrium state with increase in moisture content. The diffusion coefficient was very higher; it's may be due to higher concentration gradient at lesser moisture content availability at initial stage. Once if moisture content was increased in concrete matrix may be due mixing water, aggregate quantity, and pore structure may become fully filled water, in turn thus diffusion coefficient was going on reduced as time passes with increase in moisture content for in case of all mixtures type (M1-M6). The variation of water diffusion coefficient with moisture content and R^2 value for in case of different mixtures type as shown in Table 6.

Table 1 (Variable: Slump & W/C value; Constant: Compressive strength)

Mix No	Comp/mean target strength(N/mm ²)	Slump (mm)	w/c	C (Kg)	W (Kg)	FA (Kg)	CA(Kg) 10 mm	Mixture Proportions
M1	40/47.84	0-10	0.45	3.60	1.62	5.86	18.60	1:1.63:5.16
M2	40/47.84	10-30	0.44	4.35	1.92	5.62	16.88	1:1.29:3.87
M3	40/47.84	60-180	0.43	5.43	2.34	6.42	14.30	1:1.18:2.63

Table 2 (Variable: Compressive strength & W/C value; Constant: Slump)

Mix No	Comp/mean target strength(N/mm ²)	Slump (mm)	w/c	C (Kg)	W (Kg)	FA (Kg)	CA(Kg) 10 mm	Mixture Proportions
M4	25/32.84	10-30	0.50	3.84	1.92	5.98	17.04	1:1.55:4.44
M5	30/37.84	10-30	0.45	4.27	1.92	6.09	16.50	1:1.42:3.86
M6	40/47.84	10-30	0.44	4.35	1.92	5.62	16.88	1:1.29:3.87

Table 3 De-sorptivity coefficient with time

MIX ID	Co-relation Equation	R ²
M1	Ds = 111.99 (\sqrt{t}) ^{-1.142}	0.9982
M2	Ds = 129.24(\sqrt{t}) ^{-1.118}	0.9979
M3	Ds = 106.97(\sqrt{t}) ^{-1.086}	0.9989
M4	Ds = 175.91(\sqrt{t}) ^{-1.140}	0.9970
M5	Ds = 94.68(\sqrt{t}) ^{-1.072}	0.9923
M6	Ds = 110.97(\sqrt{t}) ^{-1.158}	0.9967

Table 4 Average of De-sorptivity coefficient

MIX ID	Average	Min, value	Max, value	STD
M1	8.41	0.24	41.74	11.55
M2	9.98	0.31	46.69	13.20
M3	8.80	0.31	41.10	11.60
M4	12.90	0.36	60.38	17.09
M5	8.04	0.30	36.46	10.49
M6	7.95	0.21	37.44	10.59

Table 5 Sorptivity/De-sorptivity coefficient with time

MIX ID	Co-relation Equation	R ²
M1	S/Ds = -5E.06 (\sqrt{t}) ² +0.0018 \sqrt{t}	0.9502
M2	S/Ds = -4E.06 (\sqrt{t}) ² +0.0017 \sqrt{t}	0.9945
M3	S/Ds =-4E.06 (\sqrt{t}) ² +0.0015 \sqrt{t}	0.9274
M4	S/Ds =-3E.06 (\sqrt{t}) ² +0.0016 \sqrt{t}	0.9534
M5	S/Ds =-7E.06 (\sqrt{t}) ² +0.0022 \sqrt{t}	0.9003
M6	S/Ds =-4E.06 (\sqrt{t}) ² +0.0018 \sqrt{t}	0.9512

Table 6 Variation of Water diffusion coefficient with moisture content

MIX ID	Co-relation Equation	R ²
M1	Dw = 0.2155(Mc) ² -1.5497Mc+3.3328	0.9540
M2	Dw = 0.1704(Mc) ² -1.4602Mc+3.6678	0.9481
M3	Dw = 0.2260(Mc) ² -1.5805Mc+3.3415	0.9350
M4	Dw = 2.9459(Mc) ^{-0.791}	0.9390
M5	Dw = 2.0988(Mc) ^{-0.819}	0.9297
M6	Dw = 2.0627(Mc) ^{-0.939}	0.9994

Table 7 Variation of moisture content ratio coefficient with time duration

MIX ID	Co-relation Equation	R ²
M1	$Mt/M_\infty = 3E-07(\sqrt{t})^3 - 0.0001(\sqrt{t})^2 + 0.0189(\sqrt{t}) + 0.20466$	0.9937
M2	$Mt/M_\infty = 2E-07(\sqrt{t})^3 - 0.0001(\sqrt{t})^2 + 0.0215(\sqrt{t}) + 0.2484$	0.9876
M3	$Mt/M_\infty = 3E-07(\sqrt{t})^3 - 0.0001(\sqrt{t})^2 + 0.0175(\sqrt{t}) + 0.1944$	0.9949
M4	$Mt/M_\infty = 3E-08(\sqrt{t})^3 - 1E-04(\sqrt{t})^2 + 0.0229(\sqrt{t}) + 0.3297$	0.9688
M5	$Mt/M_\infty = 3E-07(\sqrt{t})^3 - 0.0001(\sqrt{t})^2 + 0.0195(\sqrt{t}) + 0.2022$	0.9936
M6	$Mt/M_\infty = 3E-07(\sqrt{t})^3 - 0.0001(\sqrt{t})^2 + 0.0193(\sqrt{t}) + 0.196$	0.9922

Table 8 Average of Moisture content ratio coefficient

MIX ID	Average	Min, value	Max, value	STD
M1	0.098	0.022	0.188	0.066
M2	0.092	0.024	0.173	0.061
M3	0.075	0.021	0.139	0.047
M4	0.093	0.024	0.186	0.066
M5	0.090	0.025	0.185	0.059
M6	0.104	0.024	0.210	0.073

Table 9 Variation of moisture content with time

Mix ID	Average	Min, value	Max, value	STD
M1	2.61	0.44	4.19	1.41
M2	3.15	0.59	5.23	1.75
M3	2.52	0.50	4.10	1.41
M4	3.97	0.64	6.48	2.22
M5	2.75	0.55	4.45	1.53
M6	2.63	0.45	4.20	1.40

Relationship between moisture content ratio coefficient and time duration

The moisture content ratio coefficient is co-related with square root of time, in turn the average variation of moisture content ratio coefficient is slightly lesser for in case of higher compressive strength and varied slump value. But in the case of lower compressive strength and constant slump, the moisture content ratio coefficient is slightly higher for in lower compressive strength and constant slump value and goes on reduces with increased higher compressive strength and constant slump value. In fact, from this research work that, it's possible to establish tri-polynomial relationship between moisture content ratio coefficient and square root of time. It's also confirmed from the results that, the average variation of moisture content is slightly higher for in case of higher compressive strength and varied slump value. But in the case of lower compressive strength and constant slump, the moisture content is slightly higher for in lower compressive strength and constant slump value and goes on reduces with increased higher compressive strength and constant slump value. The variation of moisture content ratio coefficient with time duration and R² value for in case of different mixtures type as shown in Table 7. The variation of average moisture content with square root of time, minimum, maximum, and standard deviation for in case of different mixtures type as shown in Table 8.

Moisture content

The total amount of moisture contained within the concrete, either as water or water vapour, is known as the moisture content and is generally expressed as a percentage of the mass of the concrete. Moisture in concrete is present in the capillary pores and smaller gel pores within the concrete matrix. Moisture may exist as either water (when the concrete is wet and the pores are saturated) or as water vapour which provides a level of relative humidity within the concrete material. The amount of water vapour and hence relative humidity within the concrete may vary

significantly over time as water vapour moves in or out of the concrete in order to establish an equilibrium with the changing ambient conditions. The initial source of moisture in concrete is the mixing water that is used at the time of manufacture. Once the concrete is placed, there are numerous other sources of moisture. These include wet curing, exposure to the weather, wet subgrades (in slab-on-ground construction), condensation (either within the concrete or on the surface) and application of mortar tile bedding and other water-based adhesives.

The moisture content ranges between (0.45-0.6) % at an initial time duration to 4-4.5% at longer time duration as confirmed from different mixtures type (M1-M6). The moisture content varied linearly with an initial time duration, deviates afterwards at later time duration, and reaches equilibrium state for longer time duration. The moisture content was increased (50.05%) at time duration 5 min as when compared to an initial time duration 0 min in all mixtures type (M1-M6). Whereas the moisture content (76.86%) was predominantly increased at longer time duration (28 day). The moisture content (48.22-50.87%) at 0 min as well as (76.97-76.75%) at 28 days was little bit varied as compared to different mixtures type (M1-M3) and (M4-M6). Similarly, the moisture content was increased in mixture type (M4) for lower compressive strength with constant slump value at 5 min. Also the moisture content was going on decreased with an increased compressive strength in case of mixture type (M5). Similarly, still more increased compressive strength in mixture type (M6), moisture content was somewhat increased at 5 min. But at longer time duration at 28 days, moisture content slightly increased in all mixtures type (M4-M6) for in all grade of concrete. The variation of average moisture content, minimum, maximum, and standard deviation in concrete cubes at different time duration up to 28 days for in case of all mixture type (M1-M6) is represented as shown in Table 9.

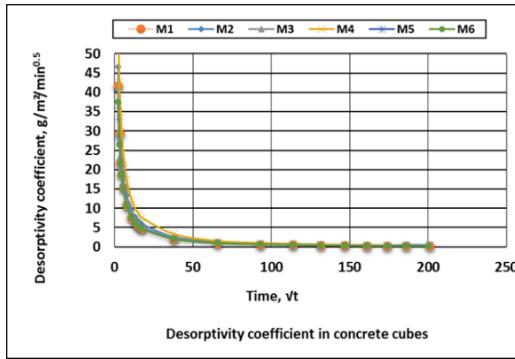


Figure 1 De-sorptivity content in concrete cubes

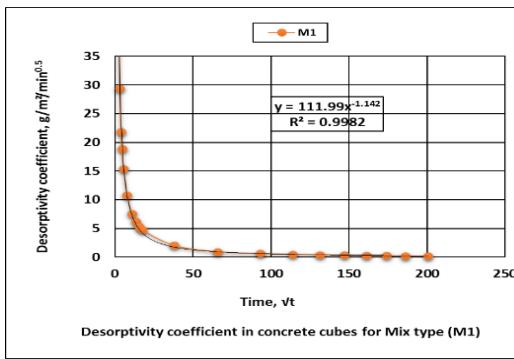


Figure 2 De-sorptivity content in concrete cubes (M1)

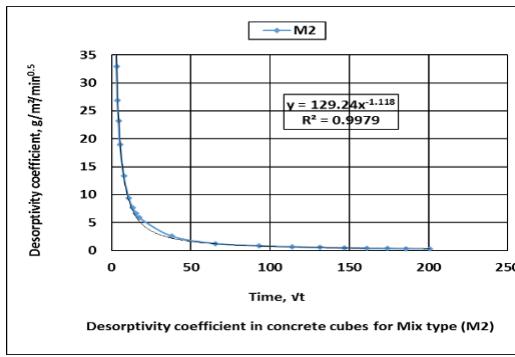


Figure 3 De-sorptivity content in concrete cubes (M2)

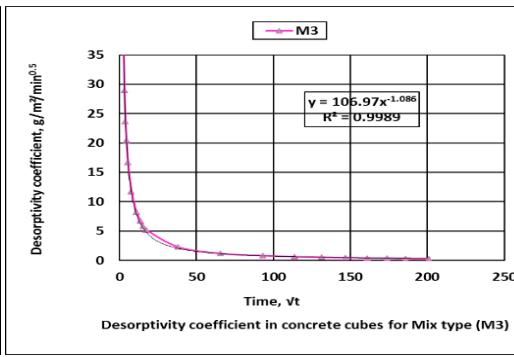


Figure 4 De-sorptivity content in concrete cubes (M3)

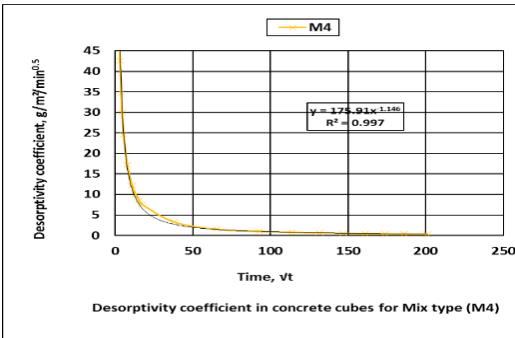


Figure 5 De-sorptivity content in concrete cubes (M4)

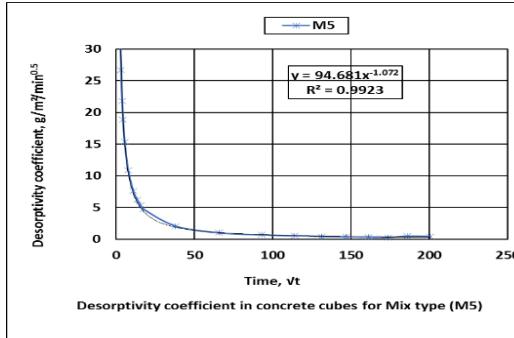


Figure 6 De-sorptivity content in concrete cubes (M5)

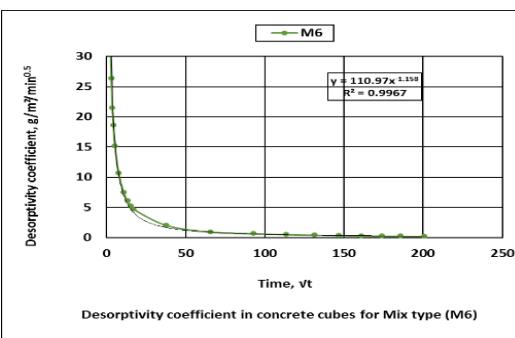
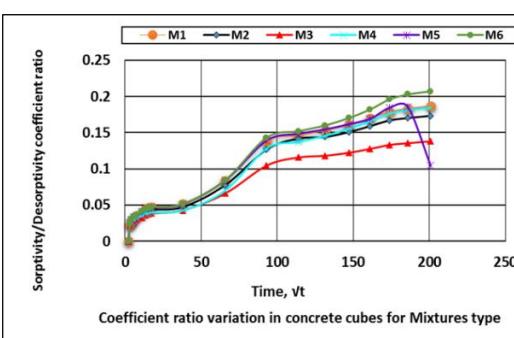


Figure 7 De-sorptivity content in concrete cubes (M6)

Figure 8 Sorptivity/De-sorptivity coefficient versus \sqrt{t}

DISCUSSION ABOUT RESULTS

The concrete infrastructures such as buildings, bridge decks and harbours, parking places, pre-stressed concrete structures, and steel structures may deteriorate due to de-icing agents, water ingress, and aggressive chemicals in the cold countries region respectively. The concrete infrastructures deterioration is considered to be as one of the major factors that could significantly change the long-term performance of concrete structures. It is well-known fact that, the deterioration rate not only depends on material compositions and construction processes, design criteria, maintenance, and protection methods but also relies on the on-going climatic environmental effect during the service phase of the concrete infrastructures lifecycle. Therefore there is a need to study water transport mechanisms with different designed mixtures type in order to assess the sorptivity-de-sorptivity coefficient in concrete structures. The present research work is made an attempt to interpret the concrete water sorptivity-de-sorptivity coefficient, de-sorptivity-time, water diffusion coefficient-sorptivity, water diffusion coefficient-moisture content, and moisture content ratio coefficient-time respectively in ordered to characterize the different concrete mixtures design for in case of concrete cubes. The variation of de-sorptivity coefficient with square root of time for in case of designed mixtures type with their correlation equation as well as R^2 values is represented in Figs.1-7 respectively. As observed from the present results that, the de-sorptivity coefficient is slightly higher for in case of constant higher concrete compressive strength and varied slump value. Whereas, the de-sorptivity coefficient is predominantly higher for in case of lower concrete compressive strength and constant slump value, but its goes on reduced with increased concrete compressive strength and constant slump value respectively.

The variation of sorptivity-de-sorptivity coefficient ratio with square root of time for in case of designed mixtures type with their correlation equation as well as R^2 values is represented in Figs.8-14 respectively. It's also observed from the present results that, the sorptivity-de-sorptivity coefficient ratio is slightly higher for in case of constant higher concrete compressive strength and varied slump value. Whereas, the de-sorptivity coefficient is slightly higher for in case of lower concrete compressive strength and constant slump value, but its goes on reduced with increased concrete compressive strength and constant slump value respectively. As observed from the results that, the sorptivity-de-sorptivity coefficient ratio is slightly increased with increased higher concrete compressive strength, and constant slump value respectively. Furthermore, the variation of sorptivity-de-sorptivity coefficient ratio is linearly varied with square root of time up to one half of long term duration and after that, its deviates from linearity proportion in all designed mixtures type respectively.

The water diffusion coefficient ratio with moisture content for in case of designed mixtures type with their correlation equation as well as R^2 values is represented in Table.6 and the variation water diffusion coefficient and moisture content is as shown in Fig.15 respectively. The water diffusion coefficient is increased at an initial stage with lesser moisture content for in case of lower compressive strength and constant slump value and goes on reduced with pre-dominantly increased moisture content. But it's also confirmed from the results that, the water diffusion coefficient is slightly decreased at initial stage with lesser moisture content and goes on reduced with lower moisture content for in case of for in case of higher compressive strength and constant slump value. Whereas in the case of constant higher compressive strength and varied slump value, the variation of water diffusion coefficient with moisture content is slightly increased at an initial stage with lower

moisture content and goes on decreases with increased moisture content for in case of constant higher compressive strength and varied slump value for in case of designed mixtures type respectively. The variation of moisture content with time duration for in case of designed mixtures type is as shown in Fig.16 respectively. The moisture content is predominantly decreased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's confirmed from the results that, the moisture content is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of moisture content with time duration is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type respectively.

CONCLUSION

The Sorptivity-de-sorptivity test was carried out on concrete cubes with water in order to evaluate the designed six mixtures type. In which in the present research work that, it's possible to establish relationship between sorptivity-de-sorptivity coefficient, de-sorptivity-time duration, water diffusion coefficient-sorptivity coefficient, water diffusion coefficient-moisture content, moisture content-time for in case of mixtures type with constant higher compressive strength and varied slump as well as for in case of mixtures type with varied compressive strength and constant slump respectively.

As from this research work that, it's possible to establish power type of equation between de-sorptivity coefficient and square root of time in designed mixtures type. The de-sorptivity coefficient is predominantly increased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's also confirmed from the results that, the de-sorptivity coefficient is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of de-sorptivity coefficient with square root of time is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type.

It's possible to establish polynomial type of equation between sorptivity-de-sorptivity coefficient ratio and square root of time in designed mixtures type. The sorptivity-de-sorptivity coefficient ratio is predominantly decreased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's also confirmed from the results that, the sorptivity-de-sorptivity coefficient ratio is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of sorptivity-de-sorptivity coefficient ratio with square root of time is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type.

From this research work that, it's possible to establish polynomial type of equation between water diffusion coefficient and moisture content with constant higher concrete compressive strength and varied slump value for in case of designed mixtures type. Finally, from this research work that, it's possible to establish power type of equation between water diffusion coefficient and moisture content with varied compressive strength and constant slump value for in case of designed mixtures type. The water diffusion coefficient is increased at an initial stage with lesser moisture content for in case of lower compressive strength and constant slump value and goes on reduced with predominantly increased moisture content. But it's also confirmed from the results that, the water diffusion coefficient is slightly decreased at initial stage with lesser moisture content and goes on reduced with lower

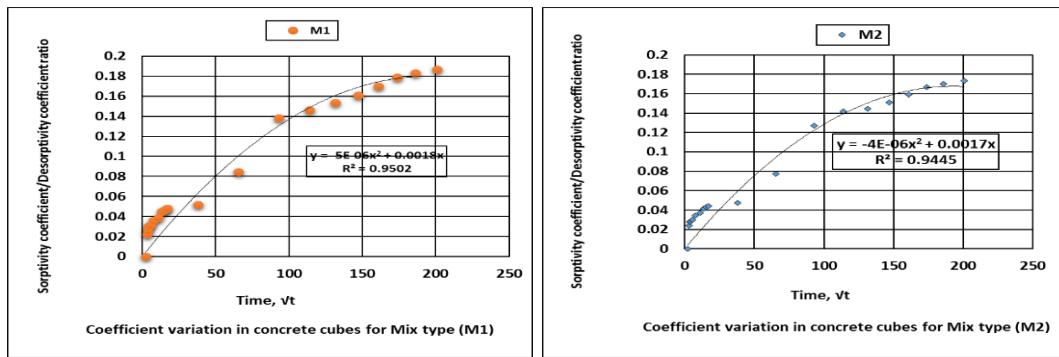
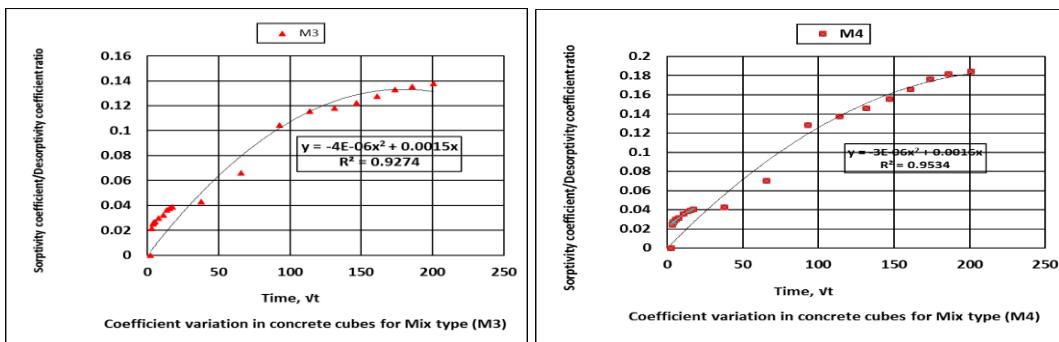
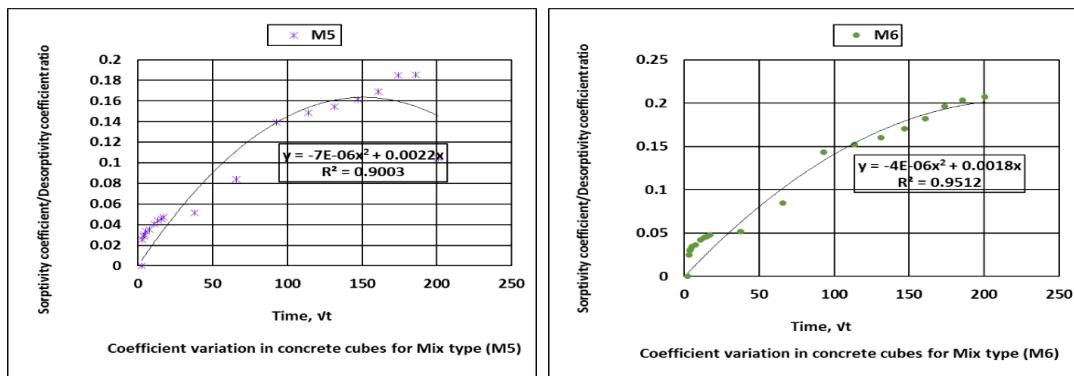
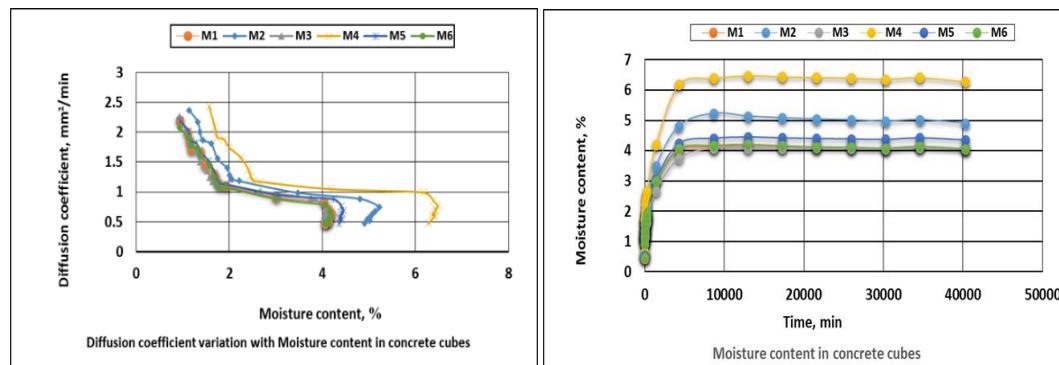
Figure 9 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 10 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 11 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 12 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 13 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 14 Sorptivity/De-sorptivity coefficient versus \sqrt{t} Figure 15 Water diffusion coefficient versus M_c

Figure 16 Moisture content in concrete cubes

moisture content for in case of higher compressive strength and constant slump value. Whereas in the case of constant higher compressive strength and varied slump value, the variation of water diffusion coefficient with moisture content is slightly increased at an initial stage with lower moisture content and goes on decreases with increased moisture content for in case of constant higher compressive strength and varied slump value for in case of designed mixtures type.

As from this research work that, it's possible to establish relationship between moisture content and time duration for in designed mixtures type. The moisture content is predominantly decreased at an initial stage as when compared to longer time duration for in case of all mixtures type. It's confirmed from the results that, the moisture content is significantly decreased for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of moisture content with time duration is slightly higher and goes on decreases with increased compressive strength for in case of designed mixtures type.

In fact, from this research work that, it's possible to establish power type of equation between water diffusion coefficient and sorptivity coefficient in designed mixtures type. The water diffusion coefficient is lesser at an initial stage when the rate of absorption (sorptivity) is lesser at an initial stage for in case of all mixtures type. It's also confirmed from the results that, the water diffusion coefficient is co-related with sorptivity coefficient, in turn the average variation of water diffusion coefficient with sorptivity coefficient is slightly more for in case of higher compressive strength and varied slump. But in the case of lower compressive strength and constant slump, the variation of water diffusion coefficient with sorptivity coefficient is slightly higher in case of lower compressive strength and constant slump and goes on decreases with increased compressive strength for in case of designed mixtures type.

REFERENCES

- Wilson M.A., Carter M.A., and Hoff W.D., 1999, British Standard and RILEM absorption tests: A critical evaluation, Materials and Structures; 32, pp. 571-578.
- Dullien F.A.L., 1992, Porous Media: Fluid Transport and Pore Structures. Second Edition, Academic Press, Inc.
- Yong R.N., Mohamed A.M.O., and Warkentin B.P., 1992, Principle of Contaminant Transport in Soils, Elsevier Science Publishers B.V.
- Boddy A., Bentz D.P., Thomas M.D.A., Hooton R.D., 1999, An overview and Sensitivity Study of a Multi-mechanistic Chloride Transport Model, Cement and Concrete Research; 29, pp. 827-837.
- Spragg, R., Castro J. Li W., Pour-Ghaz M. & Weiss J., 2011. Wetting and drying of concrete using aqueous solutions containing de-icing salts. Cement and Concrete Composites, pp. 535-542.
- Wallbank, E. J., 1989. The Performance of Concrete in Bridges. A survey of 200 Highway Bridge: HMSO.
- Lubelli, B., Hees, R.P.J van, and Groot, C.J.W.P. (2004). The role of sea salts in the occurrence of different damage mechanisms and decay patterns on brick masonry. Construction and Building Materials 18 pp119-124
- Birginie JM, Rivas T, Prieto B. 2000. Comparaison de l'alt'erabilit'e au brouillard salin de deux pierres calcaires au moyen des mesures pond'erales, acoustiques et par traitement d'images. Materiale de Construction V.50, No.259, pp. 27-43.
- Berke, N. S. & Hicks, M. C., 1991. Estimating the life cycle of reinforced concrete decks and marine piles using laboratory diffusion and corrosion data. San Diego (CA), ASTM STP 1137.
- Sutter, L., Van Dam, T., Peterson, K. R. & Johnston, D. P., 2006. Long-term effects of magnesium chloride and other concentrated salt solutions on pavement and structural Portland cement concrete:

Phase I results. Transport Research Record: Journal of the Transportation Research Board, 1979(1), pp. 60-68

- Li, W., Pour-Ghaz, M., Castro, J. & Weiss, J. W., 2012. Water Absorption and Critical Degree of Saturation Relating to Freeze-Thaw Damage in Concrete Pavement Joints. Journal of Materials in civil Engineering, pp. 299-307.
- Wang, K., Nelsen, D. E. & Nixon, W. A., 2006. Damaging effects of de-icing chemicals on concrete materials. Cement & Concrete Composites, pp.173-188.
- Wojciech Kubissa, Roman Jaskulski, Peter Koteš, and Miroslav Brodnan, Variability of sorptivity in the concrete element according to the method of compacting, Procedia Engineering, V.153, 2016, pp.355–360.
- W. Kubissa, K. Pietrzak, J. Kubissa, M. Banach, O metodach pomiaru sorpcyjno Ģci betonu, Inżynieria i Budownictwo 68 (11/2012) (2012), pp. 596–598.
- W. Kubissa, J. Kubissa, R. Jaskulski, Badanie sorpcyjno Ģci betonu w fundamencie kruszarki, in: III Forum Budowlane – Páock 2014, No.1, PPH Drukarnia Sp. z o.o. Sierpc, 2014, pp.171–182.
- Walid Deboucha,, Mohamed Nadjib Oudjit, Abderrazak Bouzid, Larbi Belagraa, Effect of incorporating blast furnace slag and natural pozzolana on compressive strength and capillary water absorption of concrete, Procedia Engineering, V.108, 2015, pp.254–261.
- Roman Jaskulski, Oliwia Amanda Waszak, Wojciech Kubissa, Model for forecasting the sorptivity of concretes with recycled concrete aggregate, Procedia Engineering, V.153, 2016, pp.240–247.
- Bo Zhou, and Zhenqian Chen, Experimental Study on the Hygrothermal Performance of Zeolite-Based Humidity Control Building Materials, International Journal of heat and technology, V.34, No.3, September 2016, pp.407-414.
- Z. P. Bažant and L. J. Najjar., Nonlinear water diffusion in non-saturated concrete, In: Matériaux et Constructions, V.5. No.1, 1972, pp.3–20.
- Y. Xi, Z. P. Bažant, L. Molina, and M. Jennings. Moisture diffusion in cementitious materials: Moisture capacity and diffusivity. In: Advanced Cement Based Materials, V.1, No.6, 1994, pp.258–266.
- D C Teychenné, R E Franklin., and H C Erntry, Design of normal concrete mixes, Second edition, BRE, 1988

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